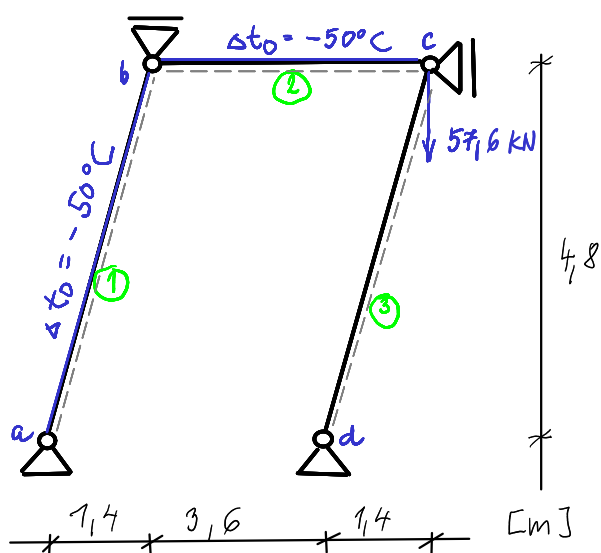


ŘEŠTE OBECNOU DEFORMAČNÍ METODOU (V MATICOVÉ NEBO SKALÁRNÍ FORMĚ) :



$$\alpha_T = 1,2 \cdot 10^{-5} \text{ K}^{-1}$$

$$E = 50 \text{ GPa} = 50 \cdot 10^6 \text{ kPa}$$



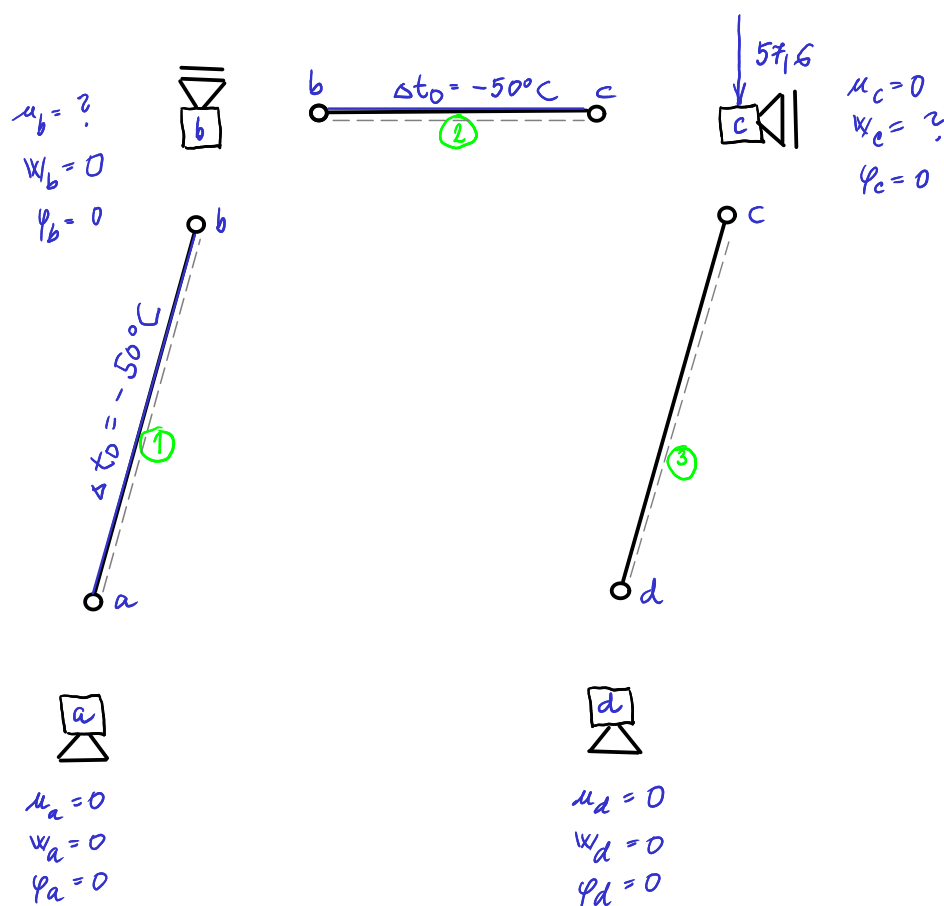
$$A = 60^2 - 40^2 = 2000 \text{ mm}^2$$

$$A = 2 \cdot 10^{-3} \text{ m}^2$$

(BUDEME DODRŽOVAT JEDNOTKY
[m, kN, kPa])

→ MATICOVÁ FORMA (KOMPLETNÍ ŘEŠENÍ)

- výpočtový model (rozdělení na tyčnický a pruty) :

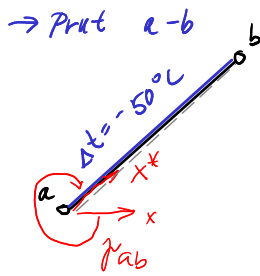


1) vektor rozměrných parametrů deformace; vektor tyčnickového zatížení

$$\{r\} = \begin{Bmatrix} u_b \\ w_c \end{Bmatrix}$$

$$\{S\} = \begin{Bmatrix} 0 \\ 57,6 \end{Bmatrix} \begin{matrix} x_b \\ z_c \end{matrix}$$

2) analýza prutu - matice tuhosti, vektory primárních koncových sil



$$l_{ab} = \sqrt{1,4^2 + 4,8^2} = 5 \text{ m}$$

$$EA = 50 \cdot 10^6 \cdot 2 \cdot 10^{-3} = 100 \cdot 10^3 \text{ kPa} \cdot \text{m}^2$$

$$(\varphi_{ab} = 286,26^\circ) \rightarrow s_{ab} = \sin \varphi_{ab} = -0,96$$

$$c_{ab} = \cos \varphi_{ab} = 0,28$$



$$[T_{ab}] = \begin{bmatrix} 0,28 & -0,96 & 0 \\ 0,96 & 0,28 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

Diagram illustrating a member of length L connecting nodes a and b . The global coordinate system (X_0, Y_0, Z_0) is shown with axes X_0 (horizontal), Y_0 (vertical), and Z_0 (depth). The local coordinate system (x, y, z) is defined along the member axis. The angle between the global X_0 axis and the local x axis is γ_{ab} . The global displacement components at node a are u_a, v_a, w_a and at node b are u_b, v_b, w_b . The local displacement components are u, v, w . The member length is L . The angle γ_{ab} is defined as $\gamma_{ab} = \cos^{-1}(c)$ and $\gamma_{ab} = \sin^{-1}(s)$.

Globální matice tuhosti

Kloub - Kloub

$$[K_{a,b}] = \begin{bmatrix} \frac{EA}{L} c^2 & \frac{EA}{L} cs & 0 & \frac{EA}{L} c^2 & \frac{EA}{L} cs & 0 \\ \frac{EA}{L} cs & \frac{EA}{L} s^2 & 0 & \frac{EA}{L} cs & \frac{EA}{L} s^2 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ \frac{EA}{L} c^2 & \frac{EA}{L} cs & 0 & \frac{EA}{L} c^2 & \frac{EA}{L} cs & 0 \\ \frac{EA}{L} cs & \frac{EA}{L} s^2 & 0 & \frac{EA}{L} cs & \frac{EA}{L} s^2 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} X_a \\ Y_a \\ Z_a \\ X_b \\ Y_b \\ Z_b \end{bmatrix}$$

$$[k_{ab}] = \begin{bmatrix} u_a & w_a & \varphi_a & u_b & w_b & \varphi_b \end{bmatrix} \begin{bmatrix} -1568 \\ 5376 \\ 0 \\ 1568 \\ -5376 \\ 0 \end{bmatrix}$$

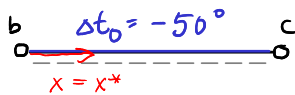
(8.14/10)
$EA\alpha_T \Delta T_0$
0
0
$-EA\alpha_T \Delta T_0$
0
0

$$\{\bar{R}_{ab}^*\} = \begin{Bmatrix} -60 \\ 0 \\ 0 \\ 60 \\ 0 \\ 0 \end{Bmatrix} \begin{Bmatrix} \bar{X}_a^* \\ \bar{Z}_a^* \\ \bar{H}_a^* \\ \bar{X}_b^* \\ \bar{Z}_b^* \\ \bar{H}_b^* \end{Bmatrix}$$

→ z LSS do GSS

$$\{\bar{R}_{ab}\} = [T_{ab}]^T \cdot \{\bar{R}_{ab}^*\} = \begin{Bmatrix} -16,8 \\ 53,6 \\ 0 \\ 16,8 \\ -53,6 \\ 0 \end{Bmatrix} \begin{Bmatrix} \bar{X}_a \\ \bar{Z}_a \\ \bar{H}_a \\ \bar{X}_b \\ \bar{Z}_b \\ \bar{H}_b \end{Bmatrix}$$

→ Prut b-c



$$l_{bc} = 5 \text{ m}$$

$$EA = 50 \cdot 10^6 \cdot 2 \cdot 10^{-3} = 100 \cdot 10^3 \text{ kPa} \cdot \text{m}^2$$

$$\varphi_{bc} = 0 \rightarrow s_{bc} = \sin \varphi_{bc} = 0$$

$$c_{bc} = \cos \varphi_{bc} = 1$$



$$[T_{bc}] = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

Kloub - Kloub

Diagram illustrating a member (beam) of length \$L\$ connecting two nodes, \$b\$ and \$c\$. The global coordinate system \$(X, Y, Z)\$ is shown with axes \$X_0, Y_0, Z_0\$. The local coordinate system \$(x, y, z)\$ is shown with axes \$x_0, y_0, z_0\$. The member is oriented at an angle \$\gamma_{ab}\$ relative to the \$X_0\$ axis. The displacement components at node \$b\$ are \$u_b, v_b, w_b\$ and at node \$c\$ are \$u_c, v_c, w_c\$. The member length is \$L\$.

$$c = \cos(\gamma_{ab})$$

$$s = \sin(\gamma_{ab})$$

Kloub - Kloub

Diagram illustrating a member (beam) of length \$L\$ connecting two nodes, \$b\$ and \$c\$. The global coordinate system \$(X, Y, Z)\$ is shown with axes \$X_0, Y_0, Z_0\$. The local coordinate system \$(x, y, z)\$ is shown with axes \$x_0, y_0, z_0\$. The member is oriented at an angle \$\gamma_{ab}\$ relative to the \$X_0\$ axis. The displacement components at node \$b\$ are \$u_b, v_b, w_b\$ and at node \$c\$ are \$u_c, v_c, w_c\$. The member length is \$L\$.

$$c = \cos(\gamma_{ab})$$

$$s = \sin(\gamma_{ab})$$

0 — 0

Diagram illustrating a member (beam) of length \$L\$ connecting two nodes, \$b\$ and \$c\$. The global coordinate system \$(X, Y, Z)\$ is shown with axes \$X_0, Y_0, Z_0\$. The local coordinate system \$(x, y, z)\$ is shown with axes \$x_0, y_0, z_0\$. The member is oriented at an angle \$\gamma_{ab}\$ relative to the \$X_0\$ axis. The displacement components at node \$b\$ are \$u_b, v_b, w_b\$ and at node \$c\$ are \$u_c, v_c, w_c\$. The member length is \$L\$.

$$c = \cos(\gamma_{ab})$$

$$s = \sin(\gamma_{ab})$$

Globální matice tuhosti

$$[K_{a,b}] = \begin{bmatrix} \frac{EA}{L} c^2 & \frac{EA}{L} cs & 0 & \frac{EA}{L} c^2 & \frac{EA}{L} cs & 0 \\ \frac{EA}{L} cs & \frac{EA}{L} s^2 & 0 & \frac{EA}{L} cs & \frac{EA}{L} s^2 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ \frac{EA}{L} c^2 & \frac{EA}{L} cs & 0 & \frac{EA}{L} c^2 & \frac{EA}{L} cs & 0 \\ \frac{EA}{L} cs & \frac{EA}{L} s^2 & 0 & \frac{EA}{L} cs & \frac{EA}{L} s^2 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

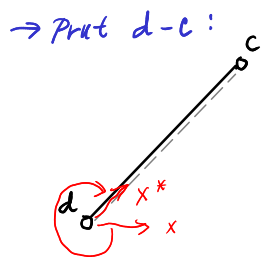
$$[k_{bc}] = \begin{bmatrix} u_b & w_b & \varphi_b & u_c & w_c & \varphi_c \end{bmatrix} \begin{bmatrix} 20000 \\ 0 \\ 0 \\ -20000 \\ 0 \\ 0 \end{bmatrix}$$

(8.14/10)
$EA\alpha_T \Delta T_0$
0
0
$-EA\alpha_T \Delta T_0$
0
0

$$\{\bar{R}_{bc}^*\} = \begin{Bmatrix} -60 \\ 0 \\ 0 \\ 60 \\ 0 \\ 0 \end{Bmatrix} \begin{Bmatrix} \bar{X}_b^* \\ \bar{Z}_b^* \\ \bar{H}_b^* \\ \bar{X}_c^* \\ \bar{Z}_c^* \\ \bar{H}_c^* \end{Bmatrix}$$

→ z LSS do GSS

$$\{\bar{R}_{bc}\} = [T_{bc}]^T \cdot \{\bar{R}_{bc}^*\} = \begin{Bmatrix} -60 \\ 0 \\ 0 \\ 60 \\ 0 \\ 0 \end{Bmatrix} \begin{Bmatrix} \bar{X}_b \\ \bar{Z}_b \\ \bar{H}_b \\ \bar{X}_c \\ \bar{Z}_c \\ \bar{H}_c \end{Bmatrix}$$



$$l_{dc} = \sqrt{1,4^2 + 4,8^2} = 5m$$

$$EA = 50 \cdot 10^6 \cdot 2 \cdot 10^{-3} = 100 \cdot 10^3 \text{ kPa} \cdot m^2$$

$$(\varphi_{dc} = 286,26^\circ) \rightarrow s_{dc} = \sin \varphi_{dc} = -0,96$$

$$c_{ds} = \cos \varphi_{dc} = 0,28$$



$$[T_{dc}] = \begin{bmatrix} 0,28 & -0,96 & 0 \\ 0,96 & 0,28 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

Diagram illustrating a member d-c (length L) in a global coordinate system X_0, Y_0 and a local coordinate system X_d, Y_d . The member is oriented at an angle γ_{dc} relative to the global X-axis. The global displacement components at node d are u_d, w_d and at node c are u_c, w_c . The local displacement components at node d are u_d, w_d and at node c are u_c, w_c . The member is labeled with E, A, I and L .

$c = \cos(\gamma_{dc})$
 $s = \sin(\gamma_{dc})$

Kloub - Kloub						○ — ○	
Globální matice tuhosti							
u_d	w_d	φ_d	u_c	w_c	φ_c		
$\frac{EA}{L} c^2$	$\frac{EA}{L} cs$	0	$\frac{EA}{L} c^2$	$-\frac{EA}{L} cs$	0	X_d	X_c
$\frac{EA}{L} cs$	$\frac{EA}{L} s^2$	0	$-\frac{EA}{L} cs$	$\frac{EA}{L} s^2$	0	Z_d	Z_c
0	0	0	0	0	0	M_d	M_c
$-\frac{EA}{L} c^2$	$-\frac{EA}{L} cs$	0	$\frac{EA}{L} c^2$	$\frac{EA}{L} cs$	0	X_b	
$\frac{EA}{L} cs$	$-\frac{EA}{L} s^2$	0	$-\frac{EA}{L} cs$	$-\frac{EA}{L} s^2$	0	Z_b	
0	0	0	0	0	0	M_b	

$$[k_{dc}] = \begin{bmatrix} u_d & w_d & \varphi_d & u_c & w_c & \varphi_c \end{bmatrix}$$

$$[k_{ab}] = \begin{bmatrix} \dots \end{bmatrix}$$

$$\{\bar{R}_{dc}^*\} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \begin{Bmatrix} \bar{X}_d^* \\ \bar{Z}_d^* \\ \bar{M}_d^* \\ \bar{X}_c^* \\ \bar{Z}_c^* \\ \bar{M}_c^* \end{Bmatrix} \rightarrow \text{z LSS do GSS}$$

$$\{\bar{R}_{dc}\} = [T_{dc}]^T \cdot \{\bar{R}_{dc}^*\} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \begin{Bmatrix} \bar{X}_d \\ \bar{Z}_d \\ \bar{M}_d \\ \bar{X}_c \\ \bar{Z}_c \\ \bar{M}_c \end{Bmatrix}$$

3) Matice modelu, vektor pravé strany $\{F\}$

$$[K] = \begin{bmatrix} 21568 & 0 \\ 0 & 18432 \end{bmatrix} \begin{Bmatrix} X_b \\ Z_c \end{Bmatrix}$$

! MATICE MUSÍ BÝT SYMETRICKÁ PODLE HLAVNÍ DIAGONÁLY (zde 0=0)

$$\{F\} = \{S\} - \{\bar{R}\} = \begin{Bmatrix} 0 \\ 57,6 \end{Bmatrix} - \begin{Bmatrix} 16,8 - 60 \\ 0 + 0 \end{Bmatrix} = \begin{Bmatrix} 43,2 \\ 57,6 \end{Bmatrix} \begin{Bmatrix} X_b \\ Z_c \end{Bmatrix}$$

4) Řešení rovnice $[K] \cdot \{r\} = \{F\} \rightarrow \{r\} = [K]^{-1} \cdot \{F\}$
nebo rozepsat soustavu rovnic

$$\begin{array}{rcl} 21568 \mu_b & + & 0 w_c = 43,2 \\ 0 \mu_b & + & 18432 w_c = 57,6 \end{array}$$

$$\{r\} = \begin{Bmatrix} \mu_b \\ w_c \end{Bmatrix} = \begin{Bmatrix} 2,003 \cdot 10^{-3} m \\ 3,125 \cdot 10^{-3} m \end{Bmatrix}$$

5) Dopolčet koncových síl $\{R\} = \{R\} + [k] \cdot \{r\}$

$$\{R_{ab}\} = \begin{Bmatrix} -16,8 \\ 57,6 \\ 0 \\ 16,8 \\ -57,6 \\ 0 \end{Bmatrix} \begin{matrix} \bar{x}_a \\ \bar{z}_a \\ \bar{M}_a \\ \bar{x}_b \\ \bar{z}_b \\ \bar{M}_b \end{matrix} + \begin{bmatrix} \mu_a & w_a & \varphi_a & \mu_b & w_b & \varphi_b \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & & & \end{bmatrix} \begin{Bmatrix} \hat{x}_a \\ \hat{z}_a \\ \hat{M}_a \\ \hat{x}_b \\ \hat{z}_b \\ \hat{M}_b \end{Bmatrix} \cdot \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 2,003 \cdot 10^{-3} \\ 0 \\ 0 \end{Bmatrix} \begin{matrix} \mu_a \\ w_a \\ \varphi_a \\ \mu_b \\ w_b \\ \varphi_b \end{matrix} = \begin{Bmatrix} -19,94 \\ 68,37 \\ 0 \\ 19,94 \\ -68,37 \\ 0 \end{Bmatrix} \begin{matrix} x_a \\ z_a \\ M_a \\ x_b \\ z_b \\ M_b \end{matrix}$$

$$\rightarrow \text{z GSS do LSS: } \{R_{ab}^*\} = [T_{ab}] \cdot \{R_{ab}\} = \begin{bmatrix} 0,28 & -0,96 & 0 & \phi & \phi & \phi \\ 0,96 & 0,28 & 0 & \phi & \phi & \phi \\ 0 & 0 & 1 & \phi & \phi & \phi \\ \phi & 0,28 & -0,96 & 0 & \phi & \phi \\ \phi & 0,96 & 0,28 & 0 & \phi & \phi \\ 0 & 0 & 0 & 1 & \phi & \phi \end{bmatrix} \begin{Bmatrix} -19,94 \\ 68,37 \\ 0 \\ 19,94 \\ -68,37 \\ 0 \end{Bmatrix} \begin{matrix} x_a \\ z_a \\ M_a \\ x_b \\ z_b \\ M_b \end{matrix} = \begin{Bmatrix} -77,22 \\ 0 \\ 0 \\ -77,22 \\ 0 \\ 0 \end{Bmatrix} \begin{matrix} x_a^* \\ z_a^* \\ M_a^* \\ x_b^* \\ z_b^* \\ M_b^* \end{matrix}$$

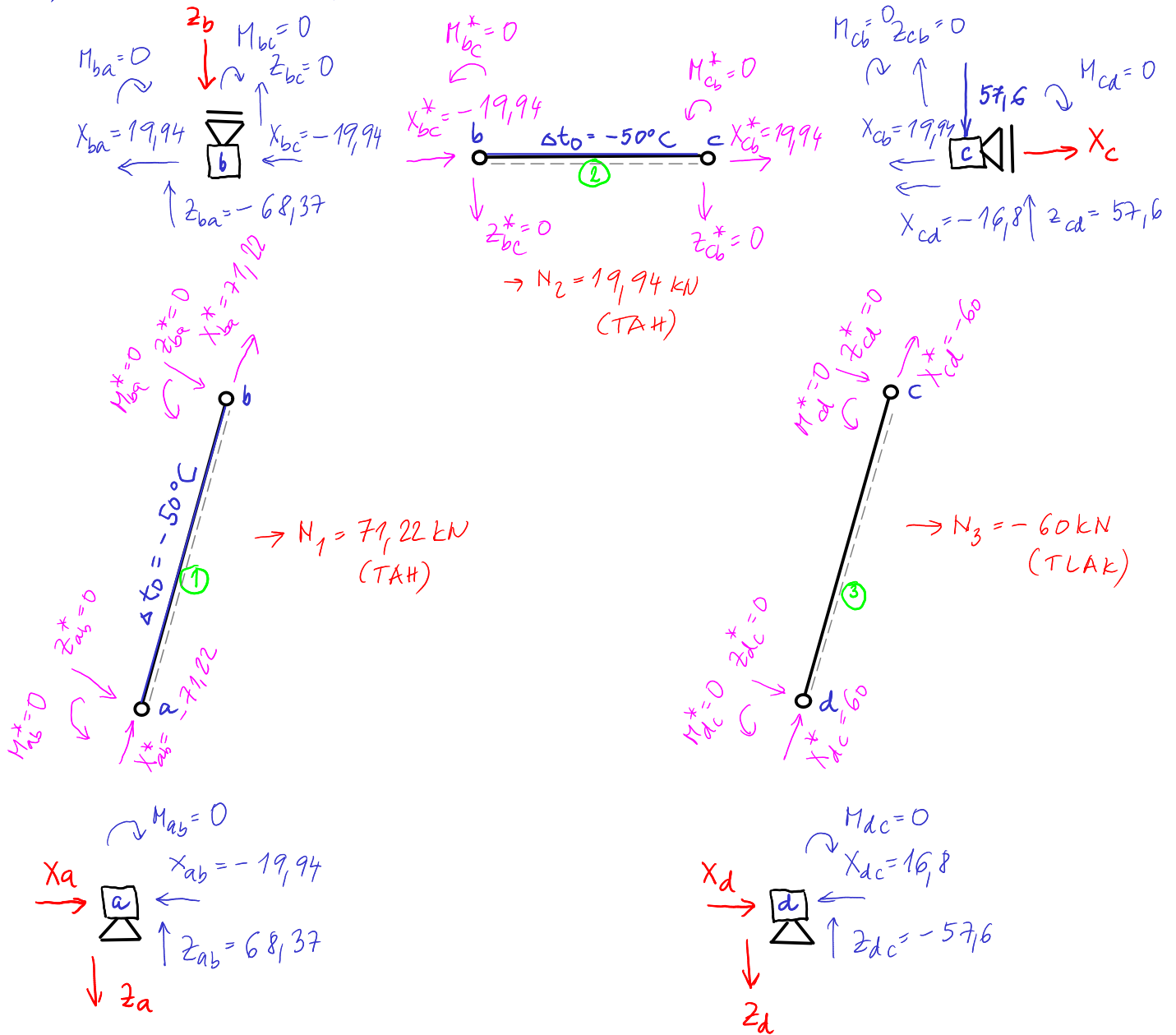
$$\{R_{bc}\} = \begin{Bmatrix} -60 \\ 0 \\ 0 \\ 60 \\ 0 \\ 0 \end{Bmatrix} \begin{matrix} \bar{x}_b \\ \bar{z}_b \\ \bar{M}_b \\ \bar{x}_c \\ \bar{z}_c \\ \bar{M}_c \end{matrix} + \begin{bmatrix} \mu_b & w_b & \varphi_b & \mu_c & w_c & \varphi_c \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & & & \end{bmatrix} \begin{Bmatrix} \hat{x}_b \\ \hat{z}_b \\ \hat{M}_b \\ \hat{x}_c \\ \hat{z}_c \\ \hat{M}_c \end{Bmatrix} \cdot \begin{Bmatrix} 2,003 \cdot 10^{-3} \\ 0 \\ 0 \\ 0 \\ 3,125 \cdot 10^{-3} \\ 0 \end{Bmatrix} \begin{matrix} \mu_b \\ w_b \\ \varphi_b \\ \mu_c \\ w_c \\ \varphi_c \end{matrix} = \begin{Bmatrix} -19,94 \\ 0 \\ 0 \\ 19,94 \\ 0 \\ 0 \end{Bmatrix} \begin{matrix} x_b \\ z_b \\ M_b \\ x_c \\ z_c \\ M_c \end{matrix}$$

$$\rightarrow \text{z GSS do LSS: } \{R_{bc}^*\} = [T_{bc}] \cdot \{R_{bc}\} = \begin{bmatrix} 1 & 0 & 0 & \phi & \phi & \phi \\ 0 & 1 & 0 & \phi & \phi & \phi \\ 0 & 0 & 1 & \phi & \phi & \phi \\ \phi & 1 & 0 & 0 & \phi & \phi \\ \phi & 0 & 1 & 0 & \phi & \phi \\ 0 & 0 & 0 & 1 & \phi & \phi \end{bmatrix} \begin{Bmatrix} -19,94 \\ 0 \\ 0 \\ 19,94 \\ 0 \\ 0 \end{Bmatrix} \begin{matrix} x_b \\ z_b \\ M_b \\ x_c \\ z_c \\ M_c \end{matrix} = \begin{Bmatrix} -19,94 \\ 0 \\ 0 \\ 19,94 \\ 0 \\ 0 \end{Bmatrix} \begin{matrix} x_b^* \\ z_b^* \\ M_b^* \\ x_c^* \\ z_c^* \\ M_c^* \end{matrix}$$

$$\{R_{dc}\} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \begin{matrix} \bar{x}_d \\ \bar{z}_d \\ \bar{M}_d \\ \bar{x}_c \\ \bar{z}_c \\ \bar{M}_c \end{matrix} + \begin{bmatrix} \mu_d & w_d & \varphi_d & \mu_c & w_c & \varphi_c \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & & & \end{bmatrix} \begin{Bmatrix} \hat{x}_d \\ \hat{z}_d \\ \hat{M}_d \\ \hat{x}_c \\ \hat{z}_c \\ \hat{M}_c \end{Bmatrix} \cdot \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 3,125 \cdot 10^{-3} \\ 0 \end{Bmatrix} \begin{matrix} \mu_d \\ w_d \\ \varphi_d \\ \mu_c \\ w_c \\ \varphi_c \end{matrix} = \begin{Bmatrix} 16,8 \\ -57,6 \\ 0 \\ -16,8 \\ 57,6 \\ 0 \end{Bmatrix} \begin{matrix} x_d \\ z_d \\ M_d \\ x_c \\ z_c \\ M_c \end{matrix}$$

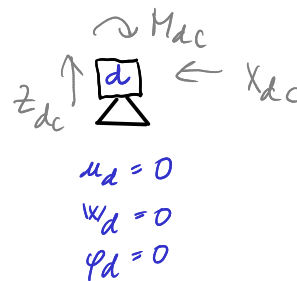
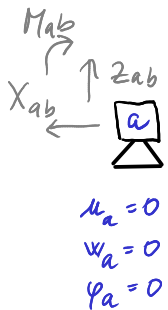
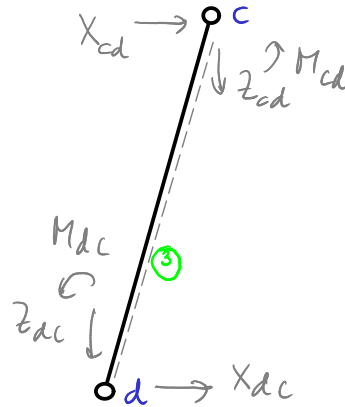
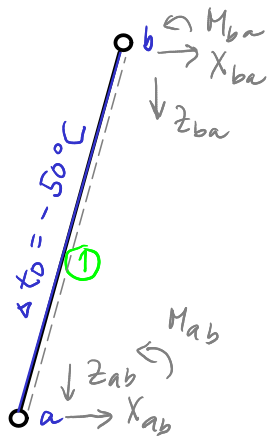
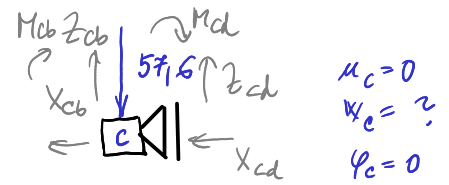
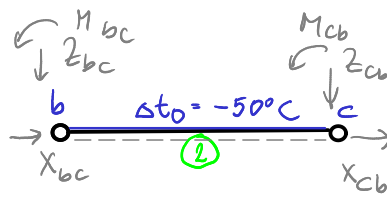
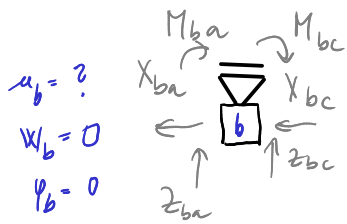
$$\rightarrow \text{z GSS do LSS: } \{R_{dc}^*\} = [T_{dc}] \cdot \{R_{dc}\} = \begin{bmatrix} 0,28 & -0,96 & 0 & \phi & \phi & \phi \\ 0,96 & 0,28 & 0 & \phi & \phi & \phi \\ 0 & 0 & 1 & \phi & \phi & \phi \\ \phi & 0,28 & -0,96 & 0 & \phi & \phi \\ \phi & 0,96 & 0,28 & 0 & \phi & \phi \\ 0 & 0 & 0 & 1 & \phi & \phi \end{bmatrix} \begin{Bmatrix} 16,8 \\ -57,6 \\ 0 \\ -16,8 \\ 57,6 \\ 0 \end{Bmatrix} \begin{matrix} x_d \\ z_d \\ M_d \\ x_c \\ z_c \\ M_c \end{matrix} = \begin{Bmatrix} 60 \\ 0 \\ 0 \\ -60 \\ 0 \\ 0 \end{Bmatrix} \begin{matrix} x_d^* \\ z_d^* \\ M_d^* \\ x_c^* \\ z_c^* \\ M_c^* \end{matrix}$$

b) kontrola rovnováhy $\rightarrow$ dopočet reakcí, vykrácení vnitřních sil



stýčnick	a	b	c	d
$\sum F_{ix} = 0:$	$x_a - x_{ab} = 0$ <u>$x_a = -19,94 \text{ kN}$</u>	$-x_{ba} - x_{bc} = 0$ ✓	$x_c - x_{cb} - x_{cd} = 0$ <u>$x_c = 3,14 \text{ kN}$</u>	$x_d - x_{dc} = 0$ <u>$x_d = 16,8 \text{ kN}$</u>
$\sum F_{iz} = 0:$	$z_a - z_{ab} = 0$ <u>$z_a = 68,37 \text{ kN}$</u>	$z_b - z_{ba} - z_{bc} = 0$ <u>$z_b = -68,37 \text{ kN}$</u>	$-z_{cb} - z_{cd} + F = 0$ ✓	$z_d - z_{dc} = 0$ <u>$z_d = -57,6 \text{ kN}$</u>
$\sum M_i = 0$	$0 = 0$ ✓	$0 + 0 = 0$ ✓	$0 + 0 = 0$ ✓	$0 = 0$ ✓

→ SKALÁRNÍ FORMA (VÝPOČET NORMÁLOVÝCH SIL)
(siny a cosiny viz maticová forma)



1) neznámé deformace + odpovídající rovnice rovnováhy ve střediscích:

$$\mu_b : \sum F_{i1b} = 0$$

$$X_{ba} + X_{bc} = 0$$

$$w_c : \sum F_{i2c} = 0$$

$$Z_{cb} + Z_{cd} - 57,6 = 0$$

2) vyjádření koncových sil pomocí tuhostí a deformací:

$$X_{ba} = N_1 \cdot c_{ab} = \frac{EA}{l_{ab}} \left[(\mu_b - 0) \cdot c_{ab} + (0 - 0) \cdot s_{ab} \right] \cdot c_{ab} - EA \alpha_T \Delta t_0 c_{ab}$$

$$X_{ba} = \frac{100 \cdot 10^3}{5} \left[0,28 \mu_b \right] \cdot 0,28 - 100 \cdot 10^3 \cdot 1,2 \cdot 10^{-5} \cdot (-50) \cdot 0,28$$

$$X_{bc} = -N_2 \cdot c_{bc} = -\frac{EA}{l_{bc}} \left[(0 - \mu_b) \cdot c_{bc} + (w_c - 0) s_{bc} \right] \cdot c_{bc} + EA \alpha_T \Delta t_0 c_{bc}$$

$$X_{bc} = -\frac{100 \cdot 10^3}{5} \left[-1 \mu_b \right] \cdot 1 + 100 \cdot 10^3 \cdot 1,2 \cdot 10^{-5} \cdot (-50) \cdot 1$$

$$Z_{cb} = N_2 \cdot s_{bc} = \frac{EA}{l_{bc}} \left[(0 - \mu_b) \cdot c_{bc} + (w_c - 0) s_{bc} \right] \cdot s_{bc} - EA \alpha_T \Delta t_0 s_{bc}$$

$$Z_{cb} = 0$$

$$Z_{cd} = N_3 \cdot s_{dc} = \frac{EA}{l_{dc}} \left[(0 - 0) \cdot c_{dc} + (w_c - 0) s_{dc} \right] \cdot s_{dc}$$

$$Z_{cd} = \frac{100 \cdot 10^3}{5} \left[-0,96 \cdot w_c \right] \cdot (-0,96)$$

3) Dosažení, úprava a řešení soustavy rovnic:

$$\mu_b: 1568 \mu_b + 16,8 + 20000 \mu_b - 60 = 0$$

$$w_c: 18432 w_c - 57,6 = 0$$

$$21568 \mu_b = 43,2$$

$$\rightarrow \mu_b = 2,003 \cdot 10^{-3} \text{ m}$$

$$18432 w_c = 57,6$$

$$\rightarrow w_c = 3,125 \cdot 10^{-3} \text{ m}$$

4) Dopačet normálových sil:

$$X_{ba} = N_1 \cdot c_{ab} = \frac{EA}{l_{ab}} \left[(\mu_b - 0) \cdot c_{ab} + (0 - 0) \cdot s_{ab} \right] \cdot c_{ab} - EA \alpha_T \Delta t_0 c_{ab}$$

$$N_1 = \frac{100 \cdot 10^3}{5} \left[2,003 \cdot 10^{-3} \cdot 0,28 \right] - 100 \cdot 10^3 \cdot 1,2 \cdot 10^{-5} \cdot (-50) = 71,22 \text{ kN}$$

$$N_1 = 71,22 \text{ kN (TAH)}$$

$$X_{bc} = -N_2 \cdot c_{bc} = -\frac{EA}{l_{bc}} \left[(0 - \mu_b) \cdot c_{bc} + (w_c - 0) s_{bc} \right] \cdot c_{bc} + EA \alpha_T \Delta t_0 c_{bc}$$

nebo

$$Z_{cb} = N_2 \cdot s_{bc} = \frac{EA}{l_{bc}} \left[(0 - \mu_b) \cdot c_{bc} + (w_c - 0) s_{bc} \right] \cdot s_{bc} - EA \alpha_T \Delta t_0 s_{bc}$$

$$N_2 = \frac{100 \cdot 10^3}{5} (-2,003 \cdot 10^{-3} \cdot 1) - 100 \cdot 10^3 \cdot 1,2 \cdot 10^{-5} \cdot (-50) = 19,94 \text{ kN}$$

$$N_2 = 19,94 \text{ kN (TAH)}$$

$$X_{dc} = -N_3 \cdot c_{dc} = -\frac{EA}{l_{dc}} \left[(\mu_c - \mu_d) c_{dc} + (w_c - w_d) s_{dc} \right] \cdot c_{dc}$$

$$-N_3 = -\frac{100 \cdot 10^3}{5} \left[3,125 \cdot 10^{-3} \cdot (-0,96) \right] = 60 \text{ kN}$$

$$N_3 = -60 \text{ kN (TLAK)}$$